

QUIZ 2
Spring 2001-2002
 (Thursday, May 23, 2002)
CVEV 051 – STRUCTURES I
CLOSED BOOK, 1 ½ HOURS

Name: _____

ID#: 1111111111111111

NOTES

- 2 PROBLEMS – 3 SHORT QUESTIONS – 11 PAGES.
- ALL YOUR ANSWERS SHOULD BE PROVIDED ON THE QUESTION SHEETS.
- **ONE EXTRA SHEET IS PROVIDED AT THE END.**
- **ASK FOR ADDITIONAL SHEETS IF YOU NEED MORE SPACE.**
- SOME ANSWERS MAY REQUIRE MUCH LESS THAN THE SPACE PROVIDED.
- **DO NOT** USE THE BACK OF THE SHEETS FOR ANSWERS.
- DRAFT BOOKLET WILL BE PROVIDED; BUT DO NOT USE FOR ANSWERS.
- BOTH QUESTION SHEETS AND DRAFT BOOKLET SHOULD BE RETURNED.

YOUR COMMENT(S)

DO NOT WRITE IN THE SPACE BELOW

MY COMMENT(S)

YOUR GRADE

Problem I: 60 /60
 Problem II: 25 /25
 Questions: 15 /15
 Other: (+2) → Trivia

TOTAL: 100 /100

Problem I: (60 points)

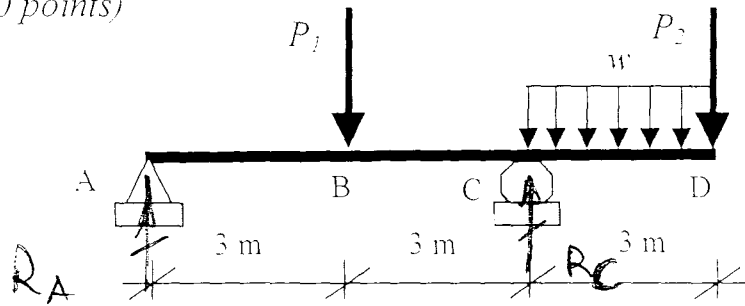


Figure I

Referring to Figure I, let $EI=50,000 \text{ kN.m}^2$ throughout the beam. Neglect the own weight of the beam.

USE THE MOMENT-AREA METHOD THROUGHOUT THIS PROBLEM.

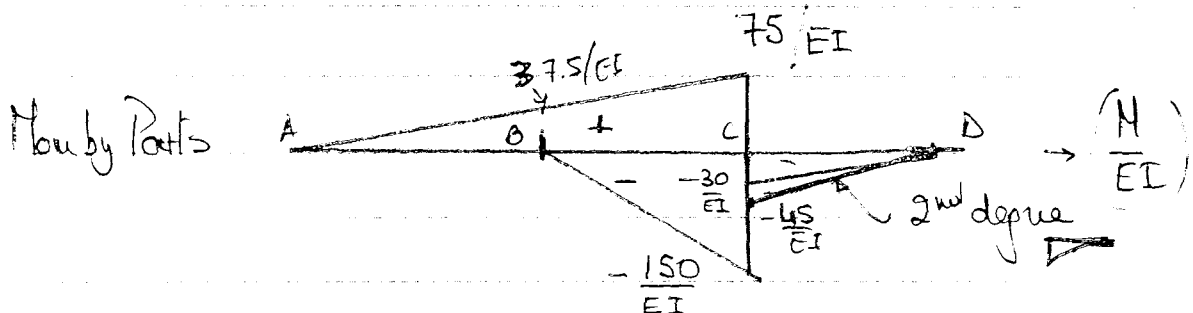
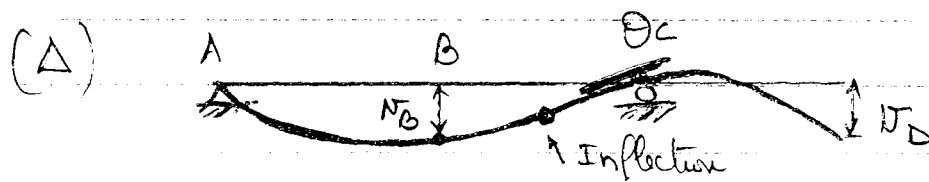
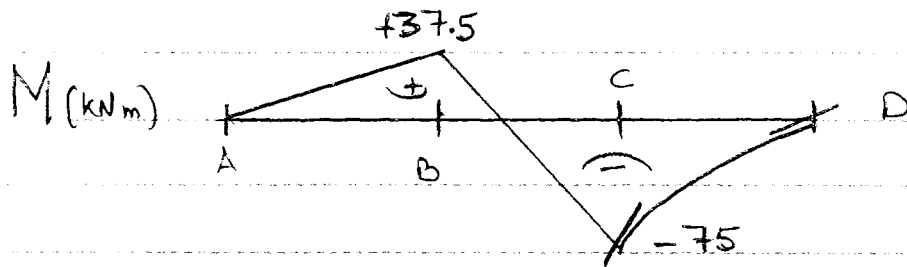
1. Let $w=10 \text{ kN/m}$, $P_1=50 \text{ kN}$, and $P_2=10 \text{ kN}$

Compute the slope at C (θ_C), the deflections at B and D (v_B and v_D), and the maximum downward deflection between A and C (v_{max}). (45 points)

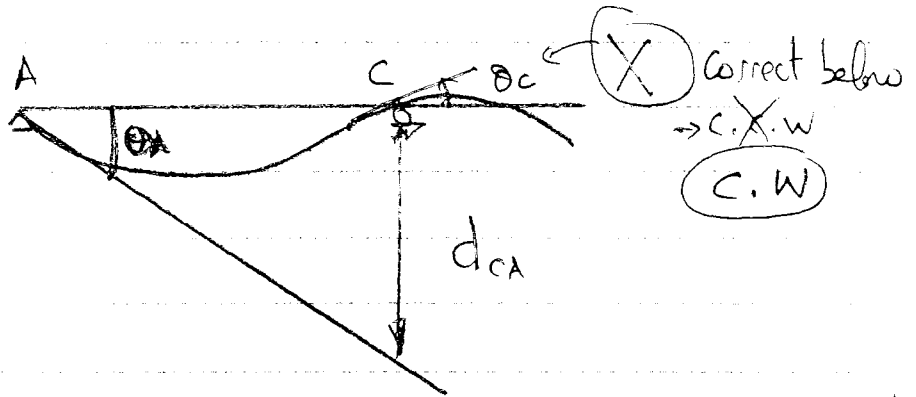
NOTE: You can calculate slopes and deflection in whichever order you find suitable.

Calculations and Diagrams: $R_A = 12.5 \text{ kN} (\uparrow)$ $R_C = 77.5 \text{ kN} (\uparrow)$

Moment Diagram: Same as Quiz #1 (S 2001-2002)



Calculations and/or Diagrams (cont'd):



$$EI d_{CA} = \left(\frac{1}{2}\right)(75 \times 6) \times \left(\frac{1}{3} \times 6\right) + \left(\frac{1}{2}\right)(-150 \times 3) \times \left(\frac{1}{3} \times 3\right) = 22.5$$

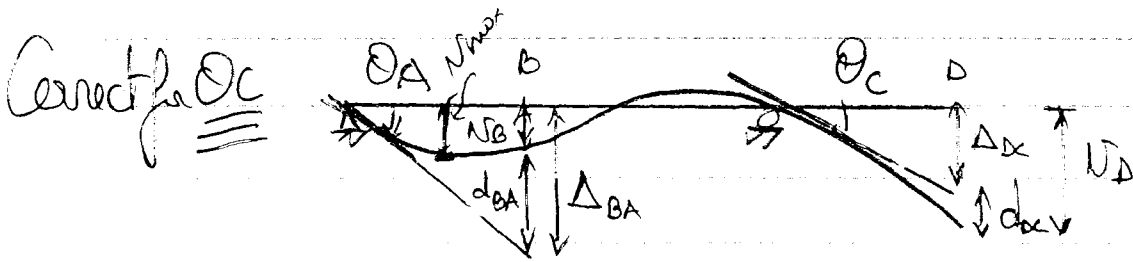
OK point above

$$d_{CA} = 4.5 \times 10^{-3} \text{ m}$$

$$\theta_A = \frac{|d_{CA}|}{6} = 0.75 \times 10^{-3} \text{ rad} \quad \theta_A = \overset{\text{clockwise}}{\ominus} 0.75 \times 10^{-3} \text{ rad}$$

$$EI(\theta_C - \theta_A) = \left(\frac{1}{2}\right)(75 \times 6) + \left(\frac{1}{2}\right)(-150 \times 3) \Rightarrow \theta_C = \theta_A = -0.75 \times 10^{-3} \text{ rad}$$

\Rightarrow clockwise



$$|\Delta_{BA}| = |\theta_A| \times 3 = 2.25 \times 10^{-3} \text{ m}$$

$$EI d_{BA} = \left(\frac{1}{2}\right)(37.5 \times 3) \times \left(\frac{1}{3} \times 3\right) = 56.25 \Rightarrow d_{BA} = 1.125 \times 10^{-3} \text{ m}$$

OK point above

$$|N_B| = |\Delta_{BA}| - |d_{BA}| = 1.125 \times 10^{-3} \text{ m} \quad \left(\downarrow\right) = 1.125 \text{ mm} = N_B$$

$$|\Delta_{DC}| = |\theta_C| \times 3 = 2.25 \times 10^{-3} \text{ m}$$

$$EI d_{DC} = \left(\frac{1}{2}\right)(-30 \times 3) \times \left(\frac{2}{3} \times 3\right) + \left(\frac{1}{3}\right)(-45 \times 3) \times \left(\frac{3}{4} \times 3\right) = -19.125 \Rightarrow d_{DC} = 3.825 \times 10^{-3} \text{ m}$$

below

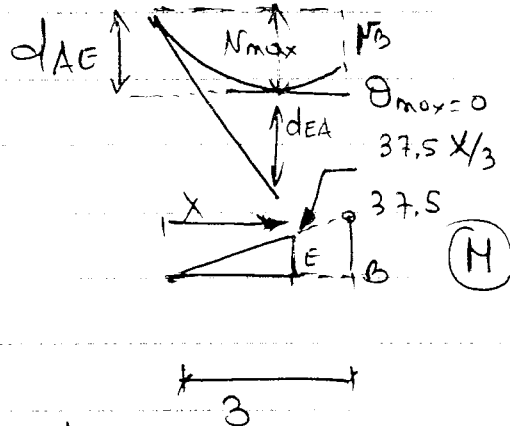
$$|N_D| = |\Delta_{DC}| + |d_{DC}| = 6.075 \times 10^{-3} \text{ m} \quad \left(\downarrow\right) = 6.075 \text{ mm} = N_D$$

Calculations and/or Diagrams (cont'd):

$N_{max}(A \rightarrow C)$ Assume between A \rightarrow B \cap E

$\theta_E = 0$

$\theta_E - \theta_A = \left(\frac{1}{2} \right) \left(\frac{37.5}{EI} \times X \times X \right)$
 $\Rightarrow X = 2.45 \text{ m} \quad (\lt 3 \text{ m})$



Con do $|N_{max}_E| = |\Delta_{EA}| - |d_{EA}|$

OR $|d_{AE}| = |N_{max}_E|$

$\Rightarrow d_{AE} = \frac{1}{2} \left(\frac{37.5}{3} \times 2.45 \times 2.45 \right) \left(\times \frac{2}{3} \times 2.45 \right) \overset{\oplus}{\text{above}} = 61.28$

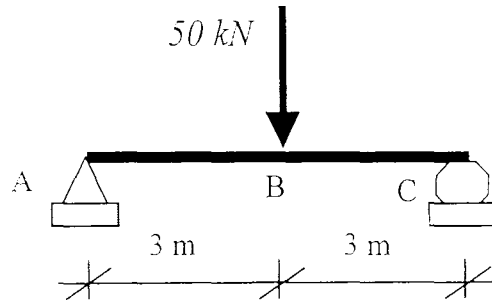
$\Rightarrow d_{AE} = 0.001226 \text{ m} \Rightarrow$

$N_{max} = N_E = 1.23 \text{ mm}$

2. Compare v_B and v_{max} from Part 1 and briefly comment. (5 points)
 Compare v_B and v_{max} from Part 1 to the mid-span deflection (v_B) of the beam (same $EI=50,000 \text{ kN.m}^2$) shown below and briefly comment. (10 points)

NOTES:

- In answering the questions above, you should use the values calculated as well as your engineering judgement; i.e. if it happens that the values obtained are not logical or do not make sense when compared with each other, this may be a hint that you may have done something wrong somewhere (?). If you do not have time to review and correct, make the proper judgement, answer accordingly, and note this in your comment.
- In calculating v_B below, you should take advantage of symmetry and of the fact that the slope is zero at B. This will simplify your calculation of v_B to a single line only (or, if you know the formula, you may use it directly).



Calculations and Diagrams:

$$\downarrow v_B = 1.125 \text{ mm}$$

$$\downarrow v_{max} = 1.226 \text{ mm}$$

$$v_{max} \text{ slightly higher than } v_B$$

$$v_{max} = \frac{PL^3}{48EI} = 4.5 \times 10^{-3} \text{ m}$$

$$v_B = \frac{1}{2} \times \left(\frac{75 \times 3}{EI} \right) \times \left(\frac{3}{3} \right) = 4.5 \times 10^{-3} \text{ m} = 4.5 \text{ mm}$$

$$50 \times 6 / 4 = 75 \text{ kNm}$$

$v_B > (v_{max})_{v_B}$
 expected since right part reduces deflection @ B

Problem II: (25 points)

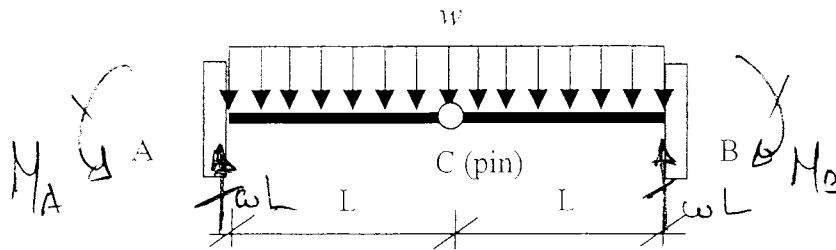


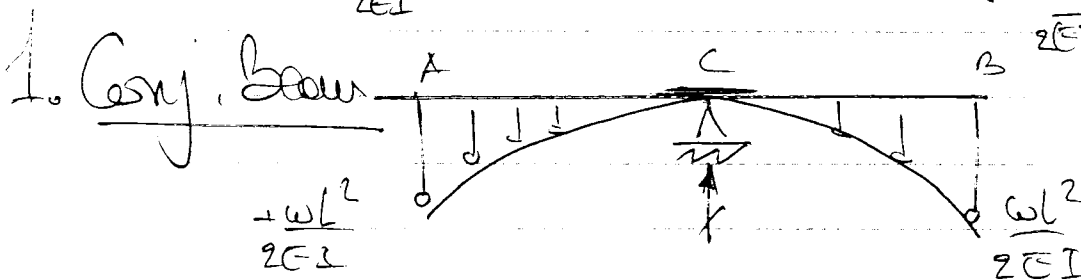
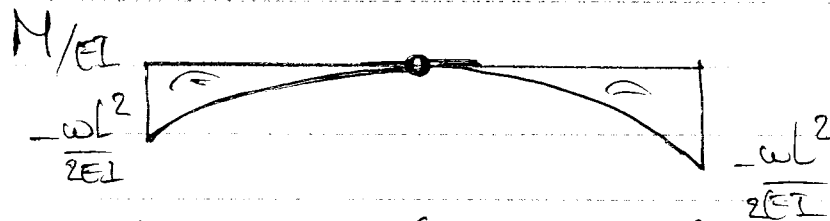
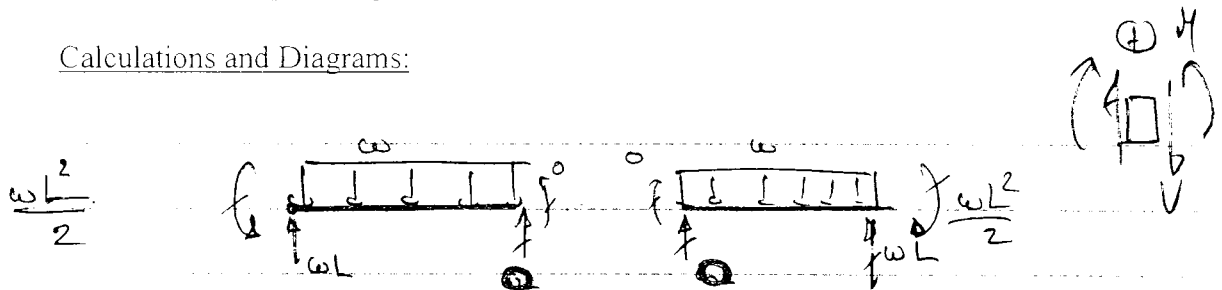
Figure II

NOTE: This should be a simple/quick problem (15 mn. maximum)

The beam shown in Figure II is SYMMETRICAL and is indeterminate to the first degree (assuming no axial forces and reactions exist in the beam). Let EI be the same throughout the beam and neglect its own weight. The vertical reaction at A is obviously wL (upward).

1. Draw the conjugate beam with the corresponding M/EI (curvature) loading. Check if this conjugate beam is stable and in equilibrium and briefly comment. (15 points)
2. Using the conjugate beam method, compute the deflection and slope at the pin and sketch the deflected shape. (10 points)

Calculations and Diagrams:

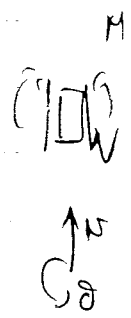
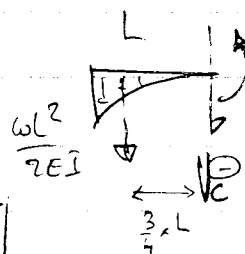


Conditionally stable under current loading
 ($\sum F_y = 0 \checkmark$ $\sum M_c = 0 \checkmark$)
 $\Rightarrow R_c = \checkmark$

Calculations and Diagrams (cont'd):

2. $M_c + \theta_c < \theta_c^-$ Note $\theta_c^- = \theta_c^+$

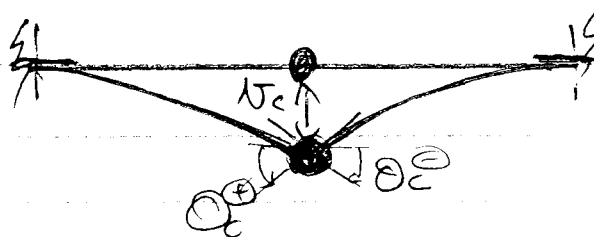
Corij: $V_c = \left(\frac{1}{3}\right) \times \left(\frac{wL^2}{2EI} \times L\right)$
 $= -\frac{wL^3}{6EI}$



Real $\theta_c^- = \frac{+wL^3}{6EI} \Rightarrow$ c.c.w
 $\Rightarrow \theta_c^+ = \frac{-wL^3}{6EI} =$ c.w.

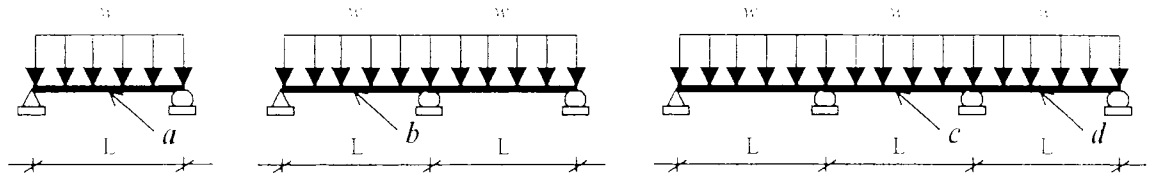
Corij: $M_c = -\frac{wL^3}{6EI} \times \frac{3L}{4} = -\frac{wL^4}{8EI}$ (down d)

Real $V_c = -\frac{wL^4}{8EI}$ (down)



NOTE: In the short questions below, EI is the same throughout, except when otherwise noted.

Question 1: (5 points)

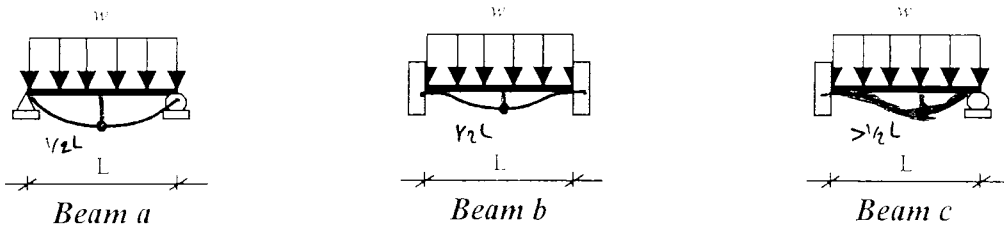


Rank the downward deflections at midspans a , b , c , and d , from smallest to largest.

$$N_c < N_b < N_d < N_a$$

Other Version
Largest to Smallest
 $a > b > d > c$

Question 2: (5 points)



Relate the maximum deflections listed below (in random order) to Beams a , b , and c . Sketch the deflection shape on the beams above, showing exactly/approximately the location of the maximum deflections.

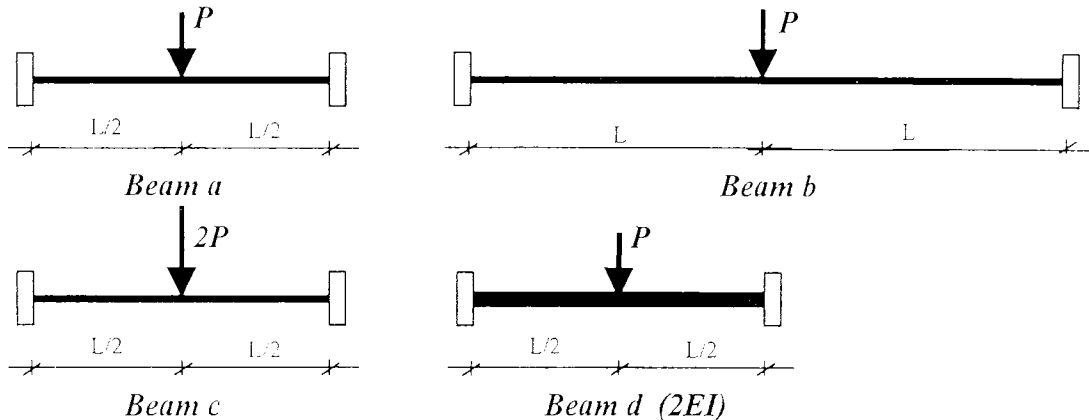
- $(1/384)wL^4/EI$
- $(5/384)wL^4/EI$
- $(1/185)wL^4/EI$

Beam: b
Beam: a
Beam: c

Other Version

c $\times/35$
a $5/384$
b $1/384$

Question 3: (5 points)



Complete/deduce the maximum deflections under the loads for Beams b , c , and d .

- Beam a : Deflection = $PL^3/192EI$
- Beam b : Deflection = $PL^3/24EI$
- Beam c : Deflection = $PL^3/96EI$
- Beam d : Deflection = $PL^3/384EI$