## Problem Set 6 Solution

Textbook: 6.15, 6.62, 6.91, 6.103, 6.118
Required Sections: 6.1, 6.2, 6.3, 6.4, 6.6 (Effect of Rough Walls and Moody Chart only), 6.7, 6.8 (no turbulent flow solution), 6.9, 6.10 (series and parallel only.)

## Problem 6.15



Figure 1: Problem 6.15
(a) $\rho=1200 \mathrm{~kg} / \mathrm{m}^{3}, \mu=6 \mathrm{~kg} / \mathrm{m} . \mathrm{s}, d=0.008 \mathrm{~m}, L=0.3 \mathrm{~m}$.

We assume that the flow is straw is operating between between (1) atmospheric pressure $p_{a}=101.4 \mathrm{kPa}$ and (2) lung pressure $p_{l}=p_{a}-3 \mathrm{kPa}$. Conservation of energy between (1) and (2)

$$
\begin{equation*}
\frac{\Delta p}{\rho g}+\Delta z=h_{f} \tag{1}
\end{equation*}
$$

where $\Delta p=p_{1}-p_{2}=p_{a}-p_{l}, \Delta z=z_{1}-z_{2}=-L$ (we assume the student is drinking the straw in a vertical position.), then

$$
\begin{equation*}
h_{f}=\frac{p_{a}-p_{l}}{\rho g}-L \tag{2}
\end{equation*}
$$

The head loss is given by

$$
\begin{equation*}
h_{f}=f \frac{L}{d} \frac{V^{2}}{2 g}=f \frac{L}{d} \frac{8 Q^{2}}{\pi^{2} d^{4} g} \tag{3}
\end{equation*}
$$

where $Q=V \frac{\pi d^{2}}{4}$ is volume flow rate.
The friction factor, assuming Laminar flow is,

$$
\begin{equation*}
f=\frac{64}{\operatorname{Re}}=\frac{64 \nu}{V d}=\frac{16 \nu \pi d}{Q} \tag{4}
\end{equation*}
$$

Combining Eqs. (3) and (4), we get

$$
Q=\frac{\pi \rho g d^{4}}{128 \mu L} h_{f}=\frac{\pi \rho g d^{4}}{128 \mu L}\left(\frac{p_{a}-p_{l}}{\rho g}-L\right)
$$

For $L=30 \mathrm{~cm}$, the volume flow rate is $Q=-0.029 \mathrm{~cm}^{3} / \mathrm{sec}$; the student would be unable to drink the milk shake.
(b) For $L=15 \mathrm{~cm}$, the volume flow rate is $Q=0.138 \mathrm{~cm}^{3} / \mathrm{sec}$. We can easily verify that $\mathrm{Re}<2000$; the flow is laminar as we assumed.

## Problem 6.62

$L=2000 \mathrm{ft}, z 2-z 1=120 \mathrm{ft}, Q=3 \mathrm{ft}^{3} / \mathrm{s}$. Pipe: Cast iron $d=6 \mathrm{in}$, pump efficiency 0.75 . Power of pump in hp?

From Table 6.1 Page 367, the surface roughness for cast iron is $\epsilon=0.00085 \mathrm{ft}$.
The energy equation between (1) and (2) is

$$
\begin{equation*}
\frac{\Delta p}{\rho g}+\Delta z+\frac{\Delta\left(V^{2}\right)}{2 g}=h_{f}+\sum h_{m}-h_{p} \tag{5}
\end{equation*}
$$

where $\Delta()=()_{1}-()_{2}, h_{f}$ is the friction head loss, and $h_{p}$ is the head delivered by the pump. The minor losses $\sum h_{m}$ are neglected in this problem.

Referring to Figure P6.62 in book, we assume (1) and (2) correspond to locations on the surfaces of the two tanks so that $p_{1}=p_{2}=p_{a}$. Then

$$
h_{p}=h_{f}-\left(z_{1}-z_{2}\right)
$$

With $h_{f}=f(L / d)\left(V^{2} / 2 g\right)$, we get

$$
h_{p}=f \frac{L}{d} \frac{V^{2}}{2 g}-\left(z_{1}-z_{2}\right)
$$

The average velocity is $V=4 Q / \pi d^{2}=15.28 \mathrm{ft} / \mathrm{s}$. For water $\mu=2.09 E-5 \mathrm{slug} / \mathrm{ft} . \mathrm{s}$ and $\rho=1.94$ slug $/ \mathrm{ft}^{3}$. Then $\operatorname{Re}_{d}=V d / \nu=7.09 \times 10^{5}$. The flow is turbulent. The friction factorcan be obtained from (a) Moody Chart Fig. 6.13 page 366, (b) Eq. 6.48 page 365, or (c) Eq. 6.49 page 366. We shall use Eq. 6.49 because it is easier to use

$$
\begin{equation*}
\frac{1}{f^{1 / 2}} \simeq-1.8 \log \left[\frac{6.9}{\operatorname{Re}_{d}}+\left(\frac{\epsilon / d}{3.7}\right)^{1.11}\right] \tag{6}
\end{equation*}
$$

leading to $f=0.02274136153, h_{f}=330 \mathrm{ft}$. Then $h_{p}=h_{f}-\left(z_{1}-z_{2}\right)=330+120=450 \mathrm{ft}$.
The power of the pump is therefore

$$
\mathcal{P}=\frac{\rho Q g h_{p}}{\xi}=204 \mathrm{hp}
$$

where $\xi=0.75$ is the pump efficiency.

## Problem 6.91

$L=0.6 \mathrm{~m}, V=2 \mathrm{~m} / \mathrm{s}$. Channel: isoceles cross section: $a=2 \mathrm{~cm}, \beta=80^{\circ}$. SAE Oil at 20 C: $\rho=870 \mathrm{~kg} / \mathrm{m}^{3}, \mu=0.104 \mathrm{~kg} / \mathrm{m} . \mathrm{s}$ from From Table A. 3 page 811.

The Perimeter of the cross section is $\mathcal{P}=2 a(1+\sin (\beta / 2))=0.06571 \mathrm{~m}$.
The cross section area is $A=a^{2} \sin (\beta / 2) \cos (\beta / 2)=0.000197 \mathrm{~m}^{2}$.
The hydraulic diameter is $d_{h}=4 A / \mathcal{P}=0.012 \mathrm{~m}$.
For Reynolds number is $\operatorname{Re}=V d_{h} / \nu=200.58$ : Laminar flow.
For Laminar flow in triangular duct with $\theta=\beta / 2=40^{\circ}$, From Table 6.4 page 383, the friction factor $f=52.9 / \operatorname{Re}=0.2637$.

The friction head loss is $h_{f}=f\left(L / d_{h}\right)\left(V^{2} / 2 g\right)=2.6933 \mathrm{~m}$.
The pressure drop is obtained from the energy equation

$$
\frac{\Delta p}{\rho g}+\Delta z=h_{f}
$$

With $\Delta z=0$, then the pressure drop is $\Delta p=\rho g h_{f}=22.96 \mathrm{kPa}$.

## Problem 6.103

From Table 6.1 Page 367, the surface roughness for cast iron is $\epsilon=0.00085 \mathrm{ft}$.
Energy equation from (1) at surface to (2) at surface according to Figure P6.103 page 436

$$
\begin{equation*}
\frac{\Delta p}{\rho g}+\Delta z+\frac{\Delta\left(V^{2}\right)}{2 g}=h_{f}+\sum h_{m}-h_{p}+h_{t} \tag{7}
\end{equation*}
$$

For $\Delta z=z_{1}-z_{2}=45 \mathrm{ft}$, pump head $h_{p}=0$, turbine head $h_{t}=0$, and $p_{1}=p_{2}=p_{a}$. Also $V_{1} \simeq V_{2}=0$ (recall that (1) and (2) are at surfaces of tanks and not correspond to flow inside pipe) then

$$
\begin{align*}
\Delta z & =h_{f}+\sum h_{m} \\
& =\frac{V_{a}^{2}}{2 g} \frac{f_{a} L_{a}}{d_{a}}+\frac{V_{b}^{2}}{2 g} \frac{f_{b} L_{b}}{d_{b}}+\frac{V_{a}^{2} K_{\text {ent }}}{2 g}+\frac{V_{a}^{2} K_{\text {exp }}}{2 g}+\frac{V_{b}^{2} K_{\text {exit }}}{2 g} \\
& =\frac{V_{a}^{2}}{2 g}\left(\frac{f_{a} L_{a}}{d_{a}}+\frac{d_{a}^{4}}{d_{b}^{4}} \frac{f_{b} L_{b}}{d_{b}}+K_{\text {ent }}+K_{\text {exp }}+\frac{d_{a}^{4}}{d_{b}^{4}} K_{\text {exit }}\right) \tag{8}
\end{align*}
$$

The subscripts $a$ and $b$ refer to the two sections of the pipe. The minor losses are sharp entrance, sudden expansion and sharp exit:

Figure 6.22 page 390 for sharp entrance (sudden contraction with $D=\infty$ ): $d / D=0$, we get $K_{\text {ent }}=0.4$.
Figure 6.22 page 390 for sudden expansion: $d / D=1 / 2$, we get $K_{\text {exp }}=0.5$.
Figure 6.22 page 390 for sharp exit (sudden expansion with $D=\infty$ ) : $d / D=0$, we get $K_{\text {exp }}=1$.

Then

$$
\begin{equation*}
\frac{V_{a}^{2}}{2 g}\left(240 f_{a}+7.5 f_{b}+0.9625\right)=45 \tag{9}
\end{equation*}
$$

We assume trubulent flow. (We will validate later).
Initial guess: fully rough flow, Moody Chart: so for $\epsilon / d_{a}=0.0102, f_{a}=0.038$ and $\epsilon / d_{b}=0.00051, f_{b}=0.0165$.

Eq. (9): $V_{a}=16.85 \mathrm{ft} / \mathrm{s}$, then $\operatorname{Re}_{a}=V_{a} d_{a} / \nu=1.303 E 5, \operatorname{Re}_{b}=V_{b} d_{b} / \nu=65171$.
Update from Eq. (6) $f_{a}=f_{a}\left(\operatorname{Re}_{a}, \epsilon / d_{a}\right)=0.03866, f_{b}=0.03188$.
Eq. (9): $V_{a}=16.629 \mathrm{ft} / \mathrm{s}$, then $\operatorname{Re}_{a}=1.286 E 5, \mathrm{Re}_{b}=64314$.
Update from Eq. (6) $f_{a}=0.03866, f_{b}=0.03189$.
The converged solution is $V_{a}=16.628 \mathrm{ft} / \mathrm{s}, Q=0.09 \mathrm{ft}^{3} / \mathrm{s}$.

