

~~Hamas~~

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$$A - M_c = \delta_{ns} M_z$$

where $\delta_{ns} = \frac{C_m}{1 - P_u / 0.75 P_c} > 1.0$
 $C_m = 0.6 + 0.4 \frac{M_1}{M_2} \geq 0.4$

$$P_c = \frac{\pi^2 EI}{(k_e l_e)^2}$$

$$EI = \frac{0.4 E_c I_g}{1 + \beta_d}$$

$$\beta_d = \frac{P_u (COL)}{P_u (COL + CL)}$$

* for column CD :

$$\beta_d = \frac{80 \times 1.4}{152.8} = 0.733$$

$$\Rightarrow EI = \frac{E_c I_g / L_e}{1 + \beta_d} = \frac{(0.12 \times 10^6 \times 3201.33) / 2.5}{1 + 0.733} = \frac{4 \times 10^9}{1.733} = 2.31 \times 10^9 \text{ lb-in}^2$$

$$\Rightarrow P_c = \frac{\pi^2 \times 2.31 \times 10^9}{(0.87 \times 18 \times 12)^2} = 744,189.74 = 744 \text{ kips}$$

$$C_m = 0.6 + 0.4(-0.4) = 0.440 > 0.4 \text{ ok}$$

$$P_u = 152.8$$

$$\therefore \delta_{ns} = \frac{0.440}{1 - \frac{152.8}{152.8}} = 0.61 < 1.0$$

\Rightarrow Take $\delta_b = 1.0$

$$M_c = 1.0 \times 107.8$$

= 107.8 ft-k (max. moment remains at end)

and design col for $P_u = 152.8$ kips

explain!

$$M_u = M_c = 107.8 \text{ ft-k}$$

check minimum eccentricity

$$e = 0.6 + 0.03h = 1.02''$$

$$\Rightarrow M_{min} = 152.8 \times 1.02 / 12 \approx 13 \text{ ft-k} < M_c \text{ ok}$$